### GENERAL NOTES:

- 1. ANY REFERENCE TO A CODE OR STANDARD SHALL BE UNDERSTOOD TO REFER TO THE LATEST EDITION AND OR REVISION OF THAT CODE OR STANDARD UNLESS NOTED OTHERWISE. 2. ALL CONSTRUCTION AND DESIGN SHALL CONFORM TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE 2012 WITH GEORGIA AMENDMENTS
- 3. CONTRACTOR SHALL COORDINATE THE LOCATION OF DEPRESSED SLABS, FLOOR DRAIN SLOPES AND EQUIPMENT PADS PRIOR TO PLACING CONCRETE
- 4. CONTRACTOR SHALL COORDINATE THE LOCATION OF MECHANICAL UNITS, ELECTRICAL FIXTURES, MECHANICAL DUCTS, DRAINS, PLUMBING EQUIPMENT AND PIPING, ETC., INCLUDING ELEVATORS AND ESCALATORS WITH ALL TRADES AFFECTED AND EQUIPMENT PURCHASED PRIOR TO PROCEEDING WITH STRUCTURAL WORK, MODIFICATIONS OR CHANGES AS A RESULT OF THIS COORDINATION SHALL BE CLEARLY DETAILED, NOTED AND SUBMITTED WITH SHOP DRAWINGS FOR REVIEW BY THE ENGINEER
- 5. CONTRACTOR SHALL VERIFY THE TYPE, SIZE, LOCATION AND NUMBER OF OPENINGS, SLEEVES, CONDUITS, EMBEDDED ITEMS, UTILITY PIPES, ETC. PRIOR TO PLACING CONCRETE OR STARTING WALL CONSTRUCTION.
- 6. UNLESS OTHERWISE SHOWN ON PLANS, THE LOCATIONS OF CONSTRUCTION JOINTS ARE SUBJECT TO PRIOR APPROVAL BY THE ENGINEER.
- 7. CONSTRUCTION CLASSIFICATION TYPE II-B.
- 8. --- INDICATES SPAN OF STEEL DECK AND/OR CONCRETE SLABS.

#### 9. ABBREVIATIONS:

@	)	_	AT	HORIZ	_	HORIZONTAL
Ø	)	_	DIAMETER	H.P.	_	HIGH POINT
<del>1</del>	#	_	NUMBER	H.S.	_	HEADED STUD
č	&	_	AND	H.W.S.	_	HEADED WELDED STU
Α.	.D.	_	AREA DRAIN	I.E.	_	FOR EXAMPLE
Αl	DD'L	_	ADDITIONAL	INFO	_	INFORMATION
Α.	.F.F.	_	ABOVE FINISHED FLOOR	INT	_	INTERIOR
Αl	LT	_	ALTERNATE	L	_	ANGLE
Αl	RCH	_	ARCHITECTURAL	L.L.H.	_	LONG LEG HORIZONT
R	M	_	RFAM	I I V	_	LONG LEG VERTICAL

LUNG LEG VERTICAL LOW POINT CENTER TO CENTER MAXIMUM C.I.P. - CAST IN PLACE MINIMUM CONSTRUCTION JOINT N.I.C. - NOT IN CONTRACT CENTER LINE NUMBER CLR – CLEAR N.S. - NEAR SIDE

C.M.U. - CONCRETE MASONRY UNIT N.T.S. - NOT TO SCALE O.C. - ON CENTER COL – COLUMN CONC - CONCRETE O.H. - OPPOSITE HAND CONTINUOUS OR CONTINUATION OPP - OPPOSITE - DEFORMED BAR ANCHOR P.C.F. - POUNDS PER CUBIC FOOT

DIAMETER P.J.F. - PREMOLDED JOINT FILLER – PLATE DWG(S) - DRAWING OR DRAWINGS PLBG - PLUMBING EXISTING CONDITIONS PSI – POUNDS PER SQUARE INCH EACH END POUNDS PER SQUARE FOOT ELEVATION R.D. – ROOF DRAIN

E.O.D. - EDGE OF DECK REINF - REINFORCE OR REINFORCEMENT E.O.S. - EDGE OF SLAB SQUARE FOOT EQ – EQUAL SIM - SIMILAR EACH WAY S.O.G. - SLAB-ON-GRADE EXP - EXPANSION SPA - SPACE OR SPACED EXTERIOR STANDARD T/ - TOP OF F.D. – FLOOR DRAIN F.S. - FAR SIDE T/F - TOP OF FOOTING FTG - FOOTING T/W - TOP OF WALL

F.V. - FIELD VERIFY TYP – TYPICAL U.N.O. - UNLESS NOTED OTHERWISE GALV - GALVANIZED VERT - VERTICAL HANGER H.C.A. - HEADED CONCRETE ANCHOR W/ – WITH

-*-*39 PSF

-- 130 KIPS

-- B

WWF - WELDED WIRE FABRIC HOOK

# DESIGN LOADS:

1. DEAD	LOADS:		
a.	ELEVATED	FLOORS:	
SLA	\RS		

MECHANICAL/ ELECTRICAL/ PLUMBING, TYPICAL CEILING/ FLOOR MISC.	8 PSF 3 PSF 4 PSF
b. ROOFS:	
ROOF SYSTEM (INCLUDING ALLOWANCE FOR RE-ROOFING) MECHANICAL/ ELECTRICAL CEILING (AS APPLICABLE) DECK MISC.	7 PSF 8 PSF 3 PSF 2 PSF 3 PSF

	DECK MISC.	2 PSF 3 PSF
	LIVE LOADS:  a. FLOORS:  UPPER FLOORS, TYPICAL (INCLUDING PARTITIONS)  ELEVATOR AND STAIR LOBBIES  STAIRS AND LANDINGS  CORRIDORS, ABOVE FIRST FLOOR  MECHANICAL ROOMS  ELEVATOR MACHINE ROOMS  ELEVATOR HOIST BEAM  b. ROOFS:	70 PSF OR 2000 L 100 PSF 100 PSF 80 PSF 150 PSF 150 PSF 5 KIPS
3.	WIND LOADS:  BASIC WIND SPEED EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT	115 MPH C ± .18
4.	SEISMIC LOADS:  SEISMIC USE GROUP  SHORT PERIOD ACC, S DS  1-SECOND PERIOD ACC, S D1  SITE CLASSIFICATION  IMPORTANCE FACTOR  ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE  PROCEDURE	III 14.0%g 10.2%g C 1.0
	0.00	4.70 14100

BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE RESPONSE MODIFICATION FACTOR

-- 3 5. SNOW LOAD: -- 5 PSF GROUND SNOW LOAD (Pg) IMPORTANCE FACTOR ( 1 s) **--** 1.0

6. DEFLECTIONS:

BASE SHEAR

SEISMIC DESIGN CATEGORY

FLOOR LIVE LOAD DEFLECTION  $\leq 1/360$ FLOOR LIVE + DEAD LOAD DEFLECTION < 1/240 ROOF LIVE LOAD DEFLECTION < 1 /360 ROOF LIVE + DEAD LOAD DEFLECTION < 1 /240

1.ALL CMU SHALL CONFORM TO ASTM C90, GRADE N, TYPE II (NORMAL WEIGHT UNITS), AND CARRY AN U.L. CLASSIFICATION TO ACHIEVE HOURLY RATINGS AS INDICATED ON

2. MINIMUM COMPRESSIVE STRENGTH (f'c) AT 28 DAYS: TYPE N MORTAR PER ASTM C270 750 PSI TYPE S MORTAR PER ASTM C270 1800 PSI 3000 PSI GROUT PER ASTM C476

3. THE DESIGN COMPRESSIVE STRENGTH OF MASONRY (f'm) BASED ON THE COMPRESSIVE STRENGTH OF MASONRY UNITS AND MORTAR TO BE USED ARE AS FOLLOWS: CONCRETE MASONRY f'm=1500 PSI4. FOR ALL WALLS AS INDICATED ON THE STRUCTURAL DRAWINGS THE MORTAR SHALL BE

"TYPE S BELOW GRADE AND TYPE N ABOVE GRADE" AND ONLY NORMAL WEIGHT UNITS SHALL BE USED IN BELOW GRADE APPLICATIONS. 5. ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID.

6. ALL CELLS WITH VERTICAL REINFORCEMENT SHALL BE GROUTED SOLID. (PROVIDE 56 BAR DIAMETER LAPS AT ALL SPLICES). TYPICAL U.N.O. 7. PROVIDE HORIZONTAL TRUSS-TYPE JOINT REINFORCEMENT AT 16" O.C. TYPICALLY, U.N.O. &

BOND BEAMS @ 8'-0"O.C. & AT TOP OF WALLS, U.N.O. JOINT REINFORCEMENT SHALL

TERMINATE AT CONTROL JOINTS. 8. PROVIDE 1#5 VERT. AT LINTEL BEARINGS, WALL INTERSECTIONS, WALL CORNERS, END OF WALLS AND EACH SIDE OF CONTROL JOINTS (U.N.O.). FILL ALL CELLS BELOW LINTEL BEARINGS SOLIDLY WITH GROUT. ALL CELLS WITH VERTICAL REÍNFORCEMENT SHALL BE GROUTED SOLID. 9. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF CONTROL JOINTS AND FOR TOP OF WALL

ELEVATIONS AND FOR LOCATION OF WALL EXPANSION JOINTS. 10. SEE 1/S7.0.1 FOR LINTEL SCHEDULE 11. ALL MASONRY SHALL BE RUNNING BOND WITH VERTICAL JOINTS LOCATED AT CENTER OF UNITS IN THE ALTERNATE COURSE BELOW, TYPICAL UNLESS NOTED OTHERWISE

12. USE LOW-LIFT GROUTING PROCEDURES ONLY. GROUT LIFTS SHALL NOT EXCEED 5'-0". 13. UNLESS NOTED OTHERWISE ON PLANS, ALL CMU SHALL BE REINFORCED AS FOLLOWS: INTERIOR WALLS - 1#5 VERT. @ 48" O.C. 8" BOND BMS. 2#5 CONT.

8" EXTERIOR WALLS - 1#5 VERT. @ 32" 3" BOND BMS. 2#5 CONT. 14. SEE 13/S 5.0.1 FOR TYPICAL CMU WALL REINFORCEMENT.

1. ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION

(AISC), 14TH EDITION. 2. UNLESS SHOWN OTHERWISE, ALL BEAM OR GIRDER CONNECTIONS SHALL BE FRAMET "MANUAL OF STEEL CONSTRUCTION. CONNECTIONS AS SHOWN IN PART 10 OF THE AISC THE DESIGN OF CONNECTIONS FOR ANY PART OF THE STRUCTURE NOT INDICATED ON THE DESIGN DRAWINGS SHALL BE COMPLETED BY THE FABRICATOR. UNLESS GREATER REACTIONS ARE INDICATED ON THE DESIGN DRAWINGS, THE CONNECTION AT EACH END OF THE MEMBER SHALL DEVELOP AT LEAST 70% OF THE TOTAL UNIFORM LOAD CAPACITY OF THE MEMBER OR A MINIMUM REACTION OF 6 KIPS, WHICHEVER IS GREATER.

IN NO CASE SHALL THE LENGTH OF FRAMED CONNECTIONS BE LESS THAN ONE—HALF OF DISTANCE OF THE MEMBER WEB. ALL STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN TH STATE OF GEORGIA TO RESIST FORCES SPECIFIED. MINIMUM OF 2 BOLTS PER VERTICAL ROW ARE REQUIRED PER CONNECTION.

3. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING: W-SHAPES: ASTM A992; M-SHAPES, S-SHAPES AND HP-SHAPES: A36; CHANNELS: ASTM A36; ANGLES: ASTM A36, PLATES: ASTM A36, U.N.O.

4. WHERE FILLET WELDS ARE SHOWN, BUT NOT SIZED, MINIMUM SIZE OF FILLET WELDS CONFORMING TO THE AISC SPECIFICATIONS SHALL BE USED.

5. MATERIAL FOR NUTS AND WASHERS SHALL BE COMPATIBLE FOR BOLTS. 6. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE STRUCTURAL WELDING

CODE - STEEL OF THE AMERICAN WELDING SOCIETY, AWS D1.1. 7. EXCEPT AS SHOWN OTHERWISE, STRUCTURAL DESIGN OF STEEL STAIRS SHALL BE PERFORMED BY THE STAIR FABRICATOR. DESIGN LIVE LOAD = 100 P.S.F. 8. ALL SHAPED HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500

GRADE B (Fy=46 KSI). ALL ROUND HSS SHALL CONFORM TO ASTM A500, GRADE B  $(F_V=42 \text{ KSI})'$ .

9. BOLTS FOR TYPICAL STRUCTURAL CONNECTIONS SHALL BE FULLY TIGHTENED HIGH STRENGTH ASTM A325 IN TYPE N BEARING TYPE CONNECTIONS.

10. ALL ANCHOR RODS SHALL CONFORM TO ASTM F 1554, GRADE 36.

11. ALL BEAMS SHALL HAVE NATURAL CAMBER UPWARD, UNLESS NOTED OTHERWISE. (C=1") -- INDICATES 1" OF POSITIVE CAMBER.

12. ALL TRUSSES AND BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT ALL FORCE COMPONENTS ARE TRANSFERRED DIRECTLY TO THE CENTER LINES OF INTERSECTING MEMBERS. WHERE THIS IS NOT POSSIBLE, CONNECTIONS SHALL BE DESIGNED FOR ALL RESULTING ECCENTRICITIES.

13. ALL VERTICAL BRACING CONNECTION WELDS (BRACE TO GUSSET PLATE, GUSSET PLATE TO COLUMN AND PLATE TO PLATE) SHALL BE TËSTED BY THE TESTING AGENCY. 14. ALL MOMENT CONNECTIONS SHALL DEVELOP THE FULL MOMENT CAPACITY OF THE BEAM OR

GIRDER, UNLESS NOTED OTHERWISE. 15. PROVIDE  $\frac{1}{2}$ " THICK WEB STIFFENERS IN BEAMS OVER ALL COLUMNS, TWO ON EACH SIDE IN LINE WITH COLUMN FLANGES.

16. PROVIDE  $\frac{1}{4}$ " CAP PLATE WELDED ALL AROUND AT ALL EXPOSED ENDS OF TUBULAR MEMBERS. 17. DO NOT USE CUTTING TORCHES FOR CORRECTING FABRICATION ERRORS IN STRUCTURAL FRAMING.

18. ALL CONTRACTOR FURNISHED DESIGN, DETAILING, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL COMPLY WITH OSHA CONSTRUCTION SAFETY AND HEALTH STANDARDS, 29 CFR PART 1926, SUBPART R, STEEL ERECTION EFFECTIVE JANUARY 18. 2002. 19. PROVIDE TEMPORARY LATERAL BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT LATERAL BRACING AND DIAPHRAGMS (FLOOR SLABS AND ROOF DECK) ARE COMPLETELY INSTALLED. THE STRUCTURAL ELEMENTS ARE UNSTABLE UNTIL THE STRUCTURE IS COMPLETED IN ACCORDANCE WITH THE PLANS.

20. AFTER FIELD ERECTION, WELDING AND BOLTING, TOUCH—UP ALL DAMAGED OR ABRADED PAINT SURFACES INCLUDING CONNECTIONS, WELDS AND BOLTS IN ACCORD WITH THE SPECIFICATIONS.

# FOUNDATION NOTES

1. FOLLOWING COMPLETION OF FOUNDATION EXCAVATION, THE SUBGRADE SHALL BE INSPECTED BY THE GEOTECHNICAL TESTING AGENCY TO DETERMINE WHETHER OTHER SUBGRADE PREPARATIONS WILL BE REQUIRED.

2. BACKFILLING SHALL NOT BE PERFORMED WITHOUT ADEQUATE WALL BRACING, EXCEPT WHERE DIFFERENTIAL OF FILL ON EITHER SIDE OF A FOUNDATION WALL IS LESS THAN TWO FEET 2'-0") AND THE CONCRETE WALL HAS ATTAINED THE SPECIFIED STRENGTH, OR WITH AUTHORIZATION FROM THE ARCHITECT, OR AS NOTED BELOW.

3. FOR SUBGRADE PREPARATION, CONTRACTOR MUST PROVIDE GEOTECHNICAL ENGINEER'S CERTIFICATION THAT FINISHED FOUNDATION SUB-GRADES MEET BEARING CAPACITIES

LISTED IN THIS DOCUMENT. 4. ALLOWABLE SOIL BEARING PRESSURE -- 4000 PSF AIR SIDE COLUMNS A-1 THRU A-14 INTERIOR COLUMNS B-1 THRU B-14 & C-1 THRU C-14 -- 4000 PSF -- 6000 PSF LANDSIDE COLUMNS D-3 THRU D-14 -- 3000 PSF COLUMN LINE 15 -- 4000 PSF LANDSLIDE COLUMNS D-1 & D-2

ENTRANCE STRUCTURE COLUMNS -- 4000 PSF 5. CONCRETE FOR WALL FOOTINGS SHALL BE PLACED IMMEDIATELY AFTER FINAL INSPECTION AND ACCEPTANCE BY THE OWNER'S TESTING AGENCY. IN NO CASE SHALL FOOTING EXCAVATIONS BE ALLOWED TO STAND OPEN OVERNIGHT OR DURING RAIN.

6. FOUNDATION DESIGN IS BASED ON THE FOLLOWING: MATERIALS MANAGERS & ENGINEERS, INC'S REPORT ENTITLED; "SUPPLEMENTARY GEOTECHNICAL EVALUATION REPORT SOUTH CARGO WAREHOUSE "C" ADDITION HARTSFIELD-JACKSON ATLANTA INTERNATIONAL AIRPORT (H-JAIA) ATLANTA, GEORGIA". 2M PROJECT NUMBER 13-008 DATED REVISED JUNE 18, 2013 AND C.O.F. ADDENDUM LETTER DATED OCTOBER 7, 2013.

7 RETAINING WALL DESIGN: o EQUIVALENT FLUID PRESSURE - 60 PCF (AT REST) o ALLOWABLE SOIL BEARING PRESSURE - 4000 PSF

> o SAFETY FACTOR FOR OVERTURNING - 1.5 o COEFFICIENT OF PASSIVE PRESSURE - 3.5

o SOIL WEIGHT - 125 PCF o COEFFICIENT OF FRICTION - 0.40 o SAFETY FACTOR FOR SLIDING - 1.5

OF THESE WALLS.

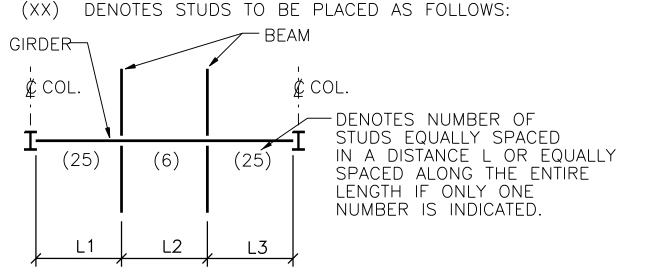
o SURCHARGE - DL=50 PSF, LL=150 PSF 8. FILL PLACED BEHIND RETAINING AND BASEMENT WALLS SHALL BE COMPACTED USING LIGHT, MANUALLY GUIDED VIBRATORY SLEDS OR ROLLERS. HEAVY COMPACTORS & OTHER CONSTRUCTION EQUIPMENT SHALL NOT BE ALLOWED TO OPERATE WITHIN EIGHT FEET (8')

9. THE UPPER 12 INCHES OF SOILS IN THE FOOTING (COLUMN AND WALL) EXCAVATION BOTTOMS SHALL BE COMPACTED TO AT LEAST 95% MODIFIED PROCTOR.

# STEEL DECK NOTES:

- 1. DESIGN, FABRICATION AND INSTALLATION OF ALL STEEL DECK SHALL BE IN ACCORD WITH STEEL DECK INSTITUTE (S.D.I. ) "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS.
- 2. SEE SPECIFICATION FOR ATTACHMENT OF STEEL DECK TO STEEL SUPPORTS, UNLESS SHOWN OTHERWISE ON PLANS.
- 3. STUDS FOR COMPOSITE FLOOR SYSTEM SHALL BE  $\frac{3}{4}$ " DIAMETER WITH AN "AFTER WELD LENGTH OF 3" FOR 11/2" DECK.

# 4. BEAMS AND GIRDERS:



b. STUDS SHALL BE PLACED DIRECTLY OVER BEAM WEB, WHERE ONLY ONE ROW OF STUDS IS REQUIRED. STUDS TO BE PLACED EQUALLY FROM EACH END TOWARD CENTER. WHERE MORE THAN ONE STUD PER RIB IS REQUIRED, INSTALL ONE STUD IN EVERY RIB AND PLACE REMAINDER OF STUDS IN RIBS STARTING AT EACH END

#### 5. MINIMUM SPACING OF STUDS IS AS FOLLOWS 4 ½ " -- PARALLEL TO BEAM

-- TRANSVERSE TO BEAM 6. THE MAXIMUM CENTER-TO-CENTER SPACING OF SHEAR STUDS SHALL NOT EXCEED (a) 8 TIMES THE TOTAL SLAB THICKNESS, (b) 3'-0" O.C.

7. CONSTRUCTION JOINTS SHALL NOT BE PLACED ON BEAM CENTERLINES. CONSTRUCTION JOINTS SHALL BE PLACED AT MID-SPAN OF THE COMPOSITE DECK SPAN, UNLESS NOTED OTHERWISE, OR APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.

1. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE (ACI ) STANDARDS 301 AND 318. 2. ALL CONCRETE REINFORCEMENT SHALL BE DEFORMED GRADE 60, AND SHALL CONFORM TO ASTM A615. 3. NOT USED

4. REINFORCEMENT SHALL NOT BE FIELD CUT WITHOUT PRIOR APPROVAL OF THE ENGINEER. 5. REINFORCEMENT LARGER THAN A #4 BAR SHALL NOT BE FIELD BENT WITHOUT PRIOR APPROVAL OF THE ENGINEER.

6. WELDING OF ASTM A615 REINFORCEMENT SHALL NOT BE PERMITTED, EXCEPT AS AUTHORIZED IN WRITING BY THE ENGINEER. ALL DEFORMED BARS WELDED TO MILD STEEL, PLATES, ANGLES OR SHAPES SHALL BE DEFORMED BAR ANCHORS (D.B.A.) CONFORMING TO ASTM A615 AND SPECIFICALLY MANUFACTURED TO BE APPLIED BY THE ELECTRIC-ARC.

7. SPLICES OF REINFORCEMENT SHALL BE MADE ONLY AS REQUIRED OR AS PERMITTED ON THE STRUCTURAL DRAWINGS OR AS AUTHORIZED BY THE ARCHITECT. ALL SPLICES SHALL BE CLASS "B" AS DEFINED BY ACL 318, UNLESS NOTED OTHERWISE.

8. REINFORCING BAR HOOKS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE STANDARD HOOKS AS DEFINED BY ACI 318, UNLESS NOTED OTHERWISE.

9. REINFORCEMENT IN SLABS OR WALLS MAY BE SHIFTED TO ALLOW FOR PLACEMENT OF INSERTS OR PENETRATIONS LESS THAN OR EQUAL TO SIX INCHES (6") IN SIZE, PROVIDED THAT MINIMUM CONCRETE COVER IS MAINTAINED.

10. OPENINGS IN SLABS ON GRADE SHALL HAVE 2 #4 PLACED ON EACH SIDE OF OPENING OR SLEEVE (MIN. 8 BARS PER OPENING OR SLËEVE). ALL BARS TO EXTEND 2'-0" (MIN.) BEYOND EACH END OF OPENING IN ALL DIRECTIONS.

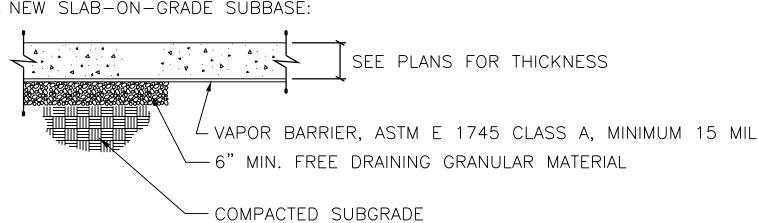
11. NO ADJACENT FLOOR OPENINGS SHALL BE LESS THAN 6" APART FROM EACH OTHER, EDGE TO EDGE. 12. CONCRETE TYPE AND USE:

28-DAY MINIMUM COMPRESSIVE SLUMP MAX. WATER/ MAXIMUM STRENGTH RANGE CEMENT RATIO WEIGHT AIR CONTENT

4000 PSI 4"±1" 150 PCF 0.45 4% ± 1% 13. TOLERANCES FOR FORMED SURFACES SHALL BE GOVERNED BY ACI 117. 14. BEFORE CONCRETE OPERATIONS BEGIN, REINFORCEMENT WHICH USES A MECHANICAL

SPLICING DEVICE, SHALL BE INSPECTED BY THE TESTING AGENCY. 15. ALL SPLICING DEVICES SHALL DEVELOP 125% OF SPECIFIED YIELD STRENGTH (Fy) OF THE BAR. 16. ALL EXPOSED CONCRETE EDGES TO HAVE  $\frac{3}{4}$  " CHAMFER. EXCEPT AS SPECIFIED FOR

ARCHITECTURAL CONCRETE 17. NEW SLAB-ON-GRADE SUBBASE:



18. SEE DETAILS 9 AND 10 ON S5.0.1 FOR TYPICAL CONCRETE WALL REINFORCEMENT.

# QUALITY/CONTROL, TESTING AND INSPECTIONS:

1. QUALITY CONTROL SUBMITTALS SHALL BE PROVIDED IN ACCORDANCE WITH THE SPECIFICATIONS. 2. QUALITY ASSURANCE INSPECTIONS AND MATERIAL TESTING SHALL BE PROVIDED BY A TESTING AGENCY. ENGAGEMENT SHALL BE BY THE OWNER (FOR A PORTION OF THE TESTING AND SPECIAL INSPECTIONS) AND THE ARCHITECT (FOR THE REMAINDER OF THE TESTING AND SPECIAL INSPECTIONS).

b. WAREHOUSE SLAB-ON-GRADE:  $F_F = 35$ ;  $F_I = 25$  (FOR MINIMUM LOCAL AND OVERALL)

# FLOOR TOLERANCES:

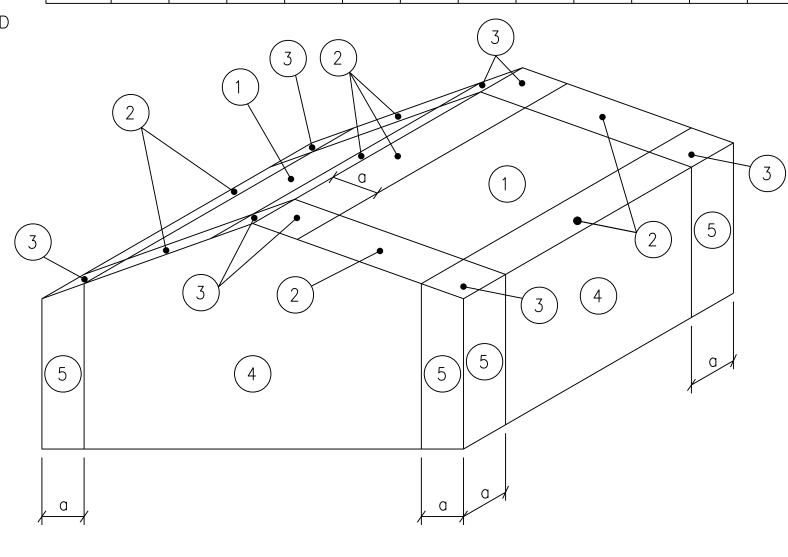
1. PROVIDE THE FOLLOWING FLOOR TOLERANCES AS MEASURED IN ACCORD WITH ASTM E 1155: a. TYPICAL SLAB-ON-GRADE:  $F_F = 25$ ;  $F_I = 20$  (FOR MINIMUM LOCAL AND OVERALL)

c. ELEVATED FLOOR FRAMING:  $F_F = 25$  (FOR MINIMUM LOCAL AND OVERALL)

### COMPONENTS AND CLADDING WIND LOADS:

- 1. POSITIVE SIGNS SIGNIFY PRESSURES ACTING TOWARD THE SURFACE. NEGATIVE SIGNS SIGNIFY PRESSURES ACTING AWAY FROM THE SURFACE.
- 2. CALCULATED WIND LOADS ARE BASED ON ASCE 7-10 REFERENCED BY THE INTERNATIONAL BUILDING CODE 2012. LINEAR INTERPOLATION IS PERMITTED FOR TRIBUTARY AREAS BETWEEN GIVEN VALUES.
- 3. "a" = 13.6'
- 4. FIGURE SHOWN IS ILLUSTRATIVE ONLY AND IS NOT INTENDED TO DEPICT THE ACTUAL STRUCTURAL DIMENSIONS.

	COMPONENTS AND CLADDING												
	EFFECTIVE WIND AREA												
ZONE	10 SF		25 SF		50	50 SF		100 SF		200 SF		500 SF	
	+	_	+	_	+	_	+	_	+	_	+	_	
1	13.4	33.0	12.3	31.9	11.5	31.1	10.6	30.2					
2	13.4	55.4	12.3	47.6	11.5	41.7	10.6	35.8					
3	13.4	83.4	12.3	64.5	11.5	50.2	10.6	35.8					
4	30.2	32.7			27.1	29.6			24.4	27.0	22.7	25.2	
5	30.2	40.3			27.1	34.1			24.4	28.7	22.7	25.2	



### STEEL JOIST NOTES:

1. ALL JOIST CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS. LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS, AS ADOPTED BY THE STEEL JOIST INSTITUTE AND AISC.

2. JOISTS SHALL BE DESIGNED AND FABRICATED BY JOIST FABRRICATOR ACCORDING TO THE STEEL JOIST INSTITUTE (S.J.I.) STANDARD SPECIFICATIONS.

3. ALL ROOF JOISTS SHALL HAVE ANGLE BOTTOM CHORDS AND SHALL BE DESIGNED FOR A NET UPLIFT EQUAL TO 5 PSF TYPICALLY IN ADDITION TO LOADS GIVEN HEREIN AND THE S.J.I. LOAD TABLES.

4. IN ADDITION TO LOADS SHOWN ON THE DRAWINGS AND THE S.J.I. LOAD TABLES, ALL MEMBERS SHALL BE DESIGNED FOR A SINGLE CONCENTRATED LOADS OF 300 LBS. PLACED AT ANY ONE PANEL ALONG THE BOTTOM CHORD, AND FOR A SINGLE CONCENTRATED LOAD OF 300 LBS. PLACED ANYWHERE ALONG THE TOP CHORD, THESE LOADS ARE TO ACT CONCURRENTLY. SEE DETAIL 2/S7.0.2 FOR REINF. WHEN LOAD DOES NOT OCCUR AT A PANEL POINT.

5. BEARING DEPTH SHALL BE 2½" FOR ALL K-SERIES JOISTS

TYPICAL, U.N.O. 6. LIVE LOAD DEFLECTION SHALL BE LESS THAN OR EQUAL TO L/360. 7. BRIDGING FOR K-SERIES JOISTS SHALL BE DESIGNED AND SPACED BY THE JOIST FABRICATOR TO MEET S.J.I. REQUIREMENTS INCLUDING UPLIFT REQUIREMENTS.

8. BRIDGING MAY BE SHIFTED TO AID THE DESIGN FOR UPLIFT. 9. DIMENSIONS OF OPENINGS AND EQUIPMENT SUPPORTS ARE SHOWN FOR BID PURPOSES ONLY. THE CONTRACTOR SHALL VERIFY THE DIMENSIONS WITH THE ACTUAL PURCHASED EQUIPMENT PRIOR TO FABRICATION, AND SHALL COORDINATE LOCATIONS OF MECHANICAL UNITS, HVAC DUCTS, PLUMBING, ELECTRICAL FIXTURES, CEILING ASSEMBLIES AND OTHER PURCHASED EQUIPMENT WITH THE VARIOUS TRADES BEFORE PROCEEDING WITH THE WORK. MODIFICATIONS AND CHANGES AS A RESULT OF THIS COORDINATION SHALL BE CLEARLY NOTED, DETAILED AND SUBMITTED FOR REVIEW ON THE SHOP

10. ALL STEEL JOIST CONNECTIONS WILL BE INSPECTED OR TESTED BY A QUALIFIED INDEPENDENT MATERIALS TESTING AGENCY IN ACCORD WITH THE SPECIFICATIONS. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED USING ONLY AWS CERTIFIED WELDING

DRAWINGS.

11. AFTER FIELD ERECTION, WELDING AND BOLTING, TOUCH—UP ALL DAMAGED PAINT SURFACES INCLUDING CONNECTIONS, WELDS AND BOLTS IN ACCORD WITH THE SPECIFICATIONS. CLEAN AREAS TO BE TOUCHED—UP IN ACCORD WITH SSPC—SP 2 OR SSPC—SP 3 PRIOR TO FIELD PAINTING.



CITY OF ATLANTA, GEORGIA

Hartsfield-Jackson

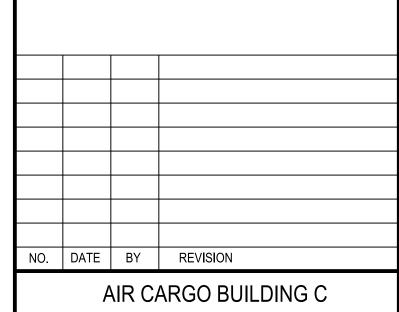


HEERY INTERNATIONAL, INC. 999 44 BROAD STREE PEACHTREE STREET, NE ATLANTA, GA 30303 ATLANTA, GA 30309 PHONE:404.522.3801 PHONE: 404.419.9190 FAX: 404.522.3823 FAX: 404.946.2017

> STEVENS & WILKINSON INC 100 PEACHTREE STREET NW. SUITE 2500 ATLANTA, GA 30303 PHONE: 404 522 8888 FAX: 404.521.6204

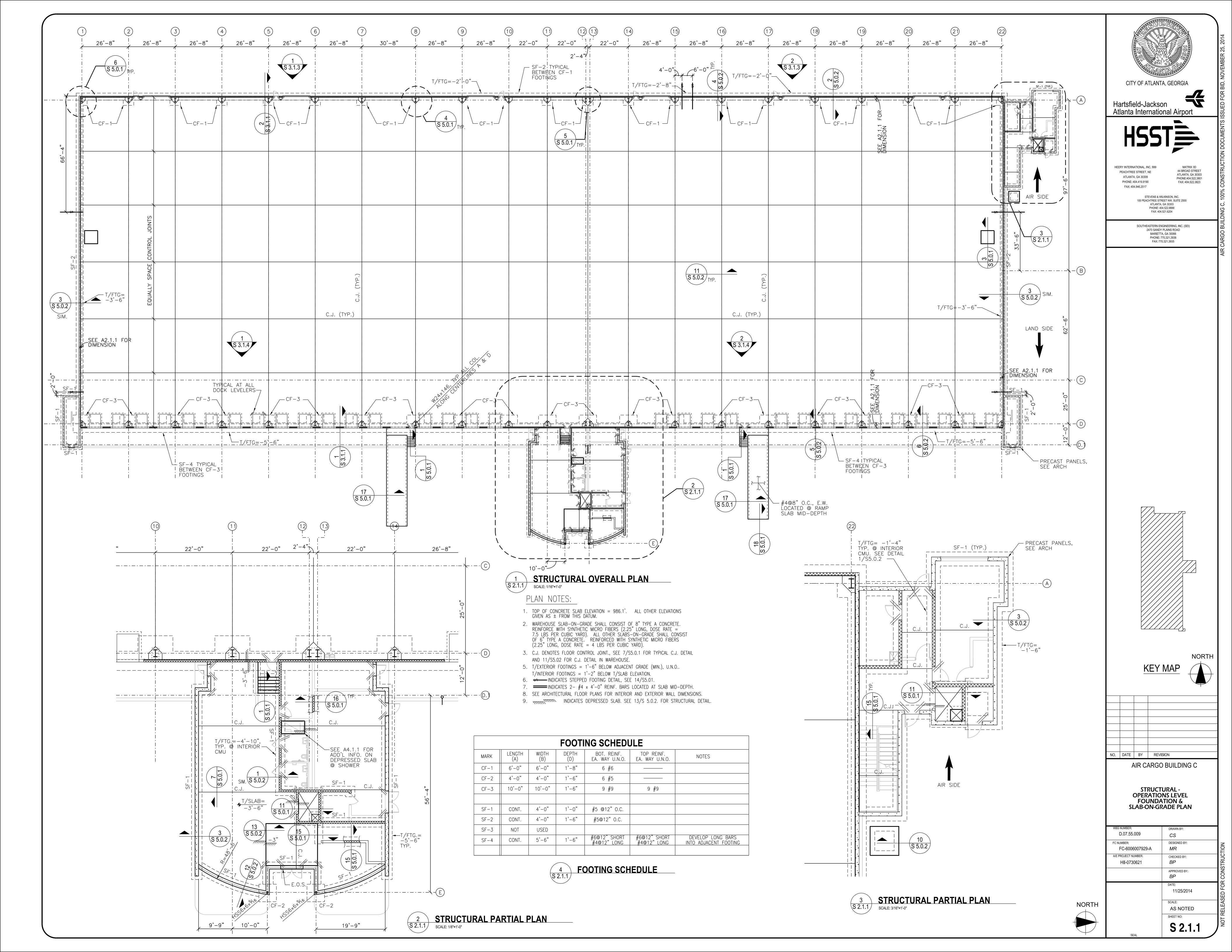
2470 SANDY PLAINS ROAD MARIETTA, GA 30066 PHONE: 770.321.3936 FAX: 770.321.3935

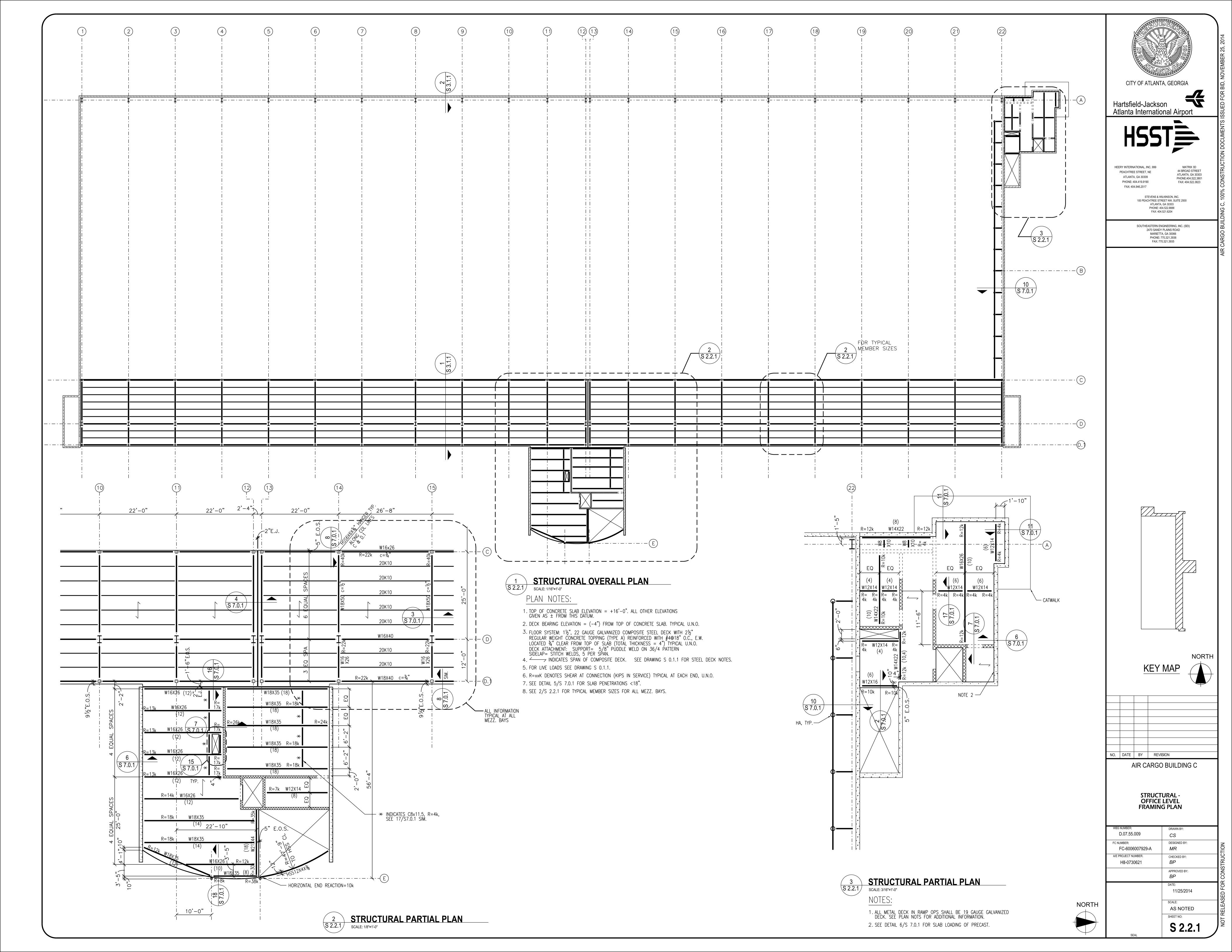
SOUTHEASTERN ENGINEERING, INC. (SEI)

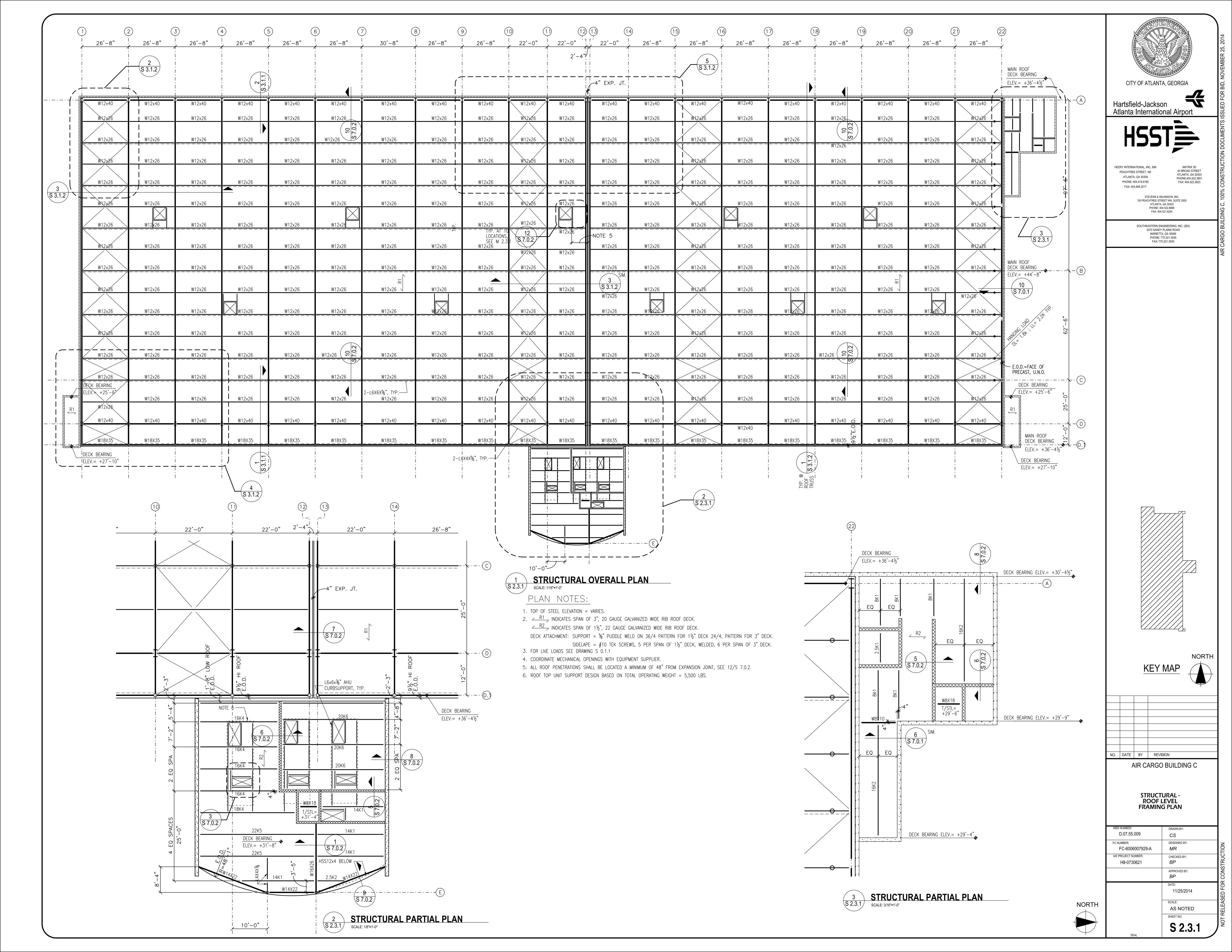


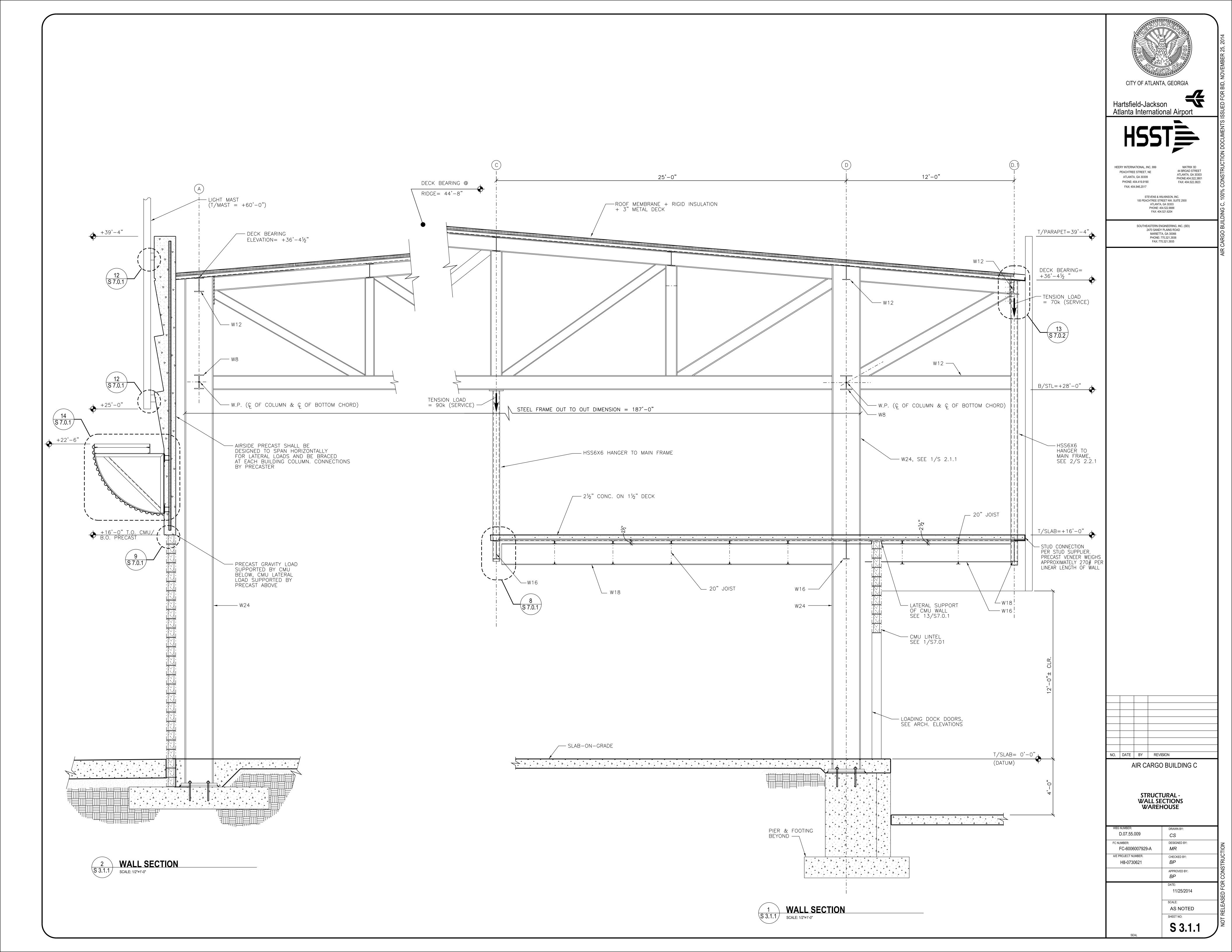
STRUCTURAL -**GENERAL NOTES &** LEGEND

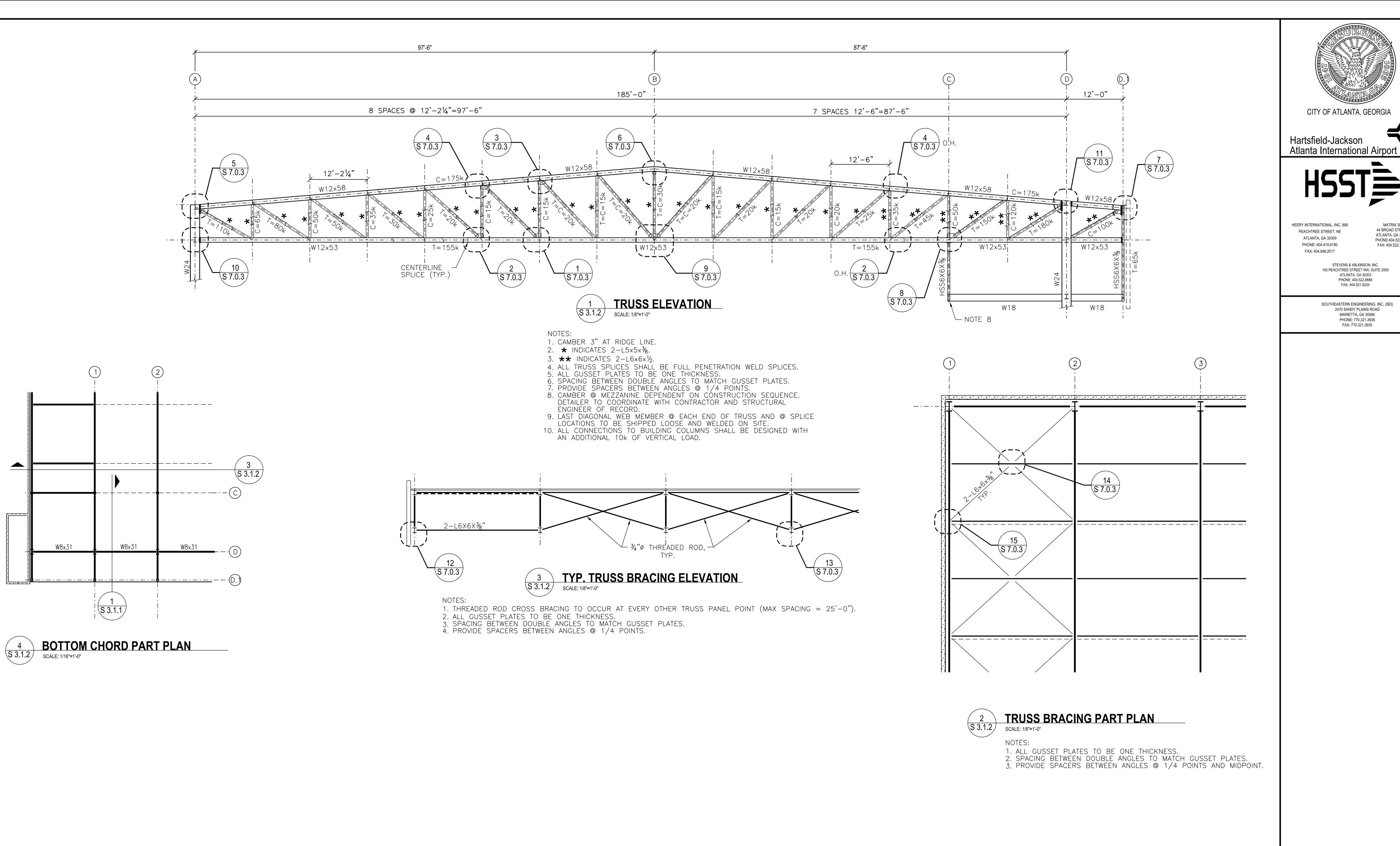
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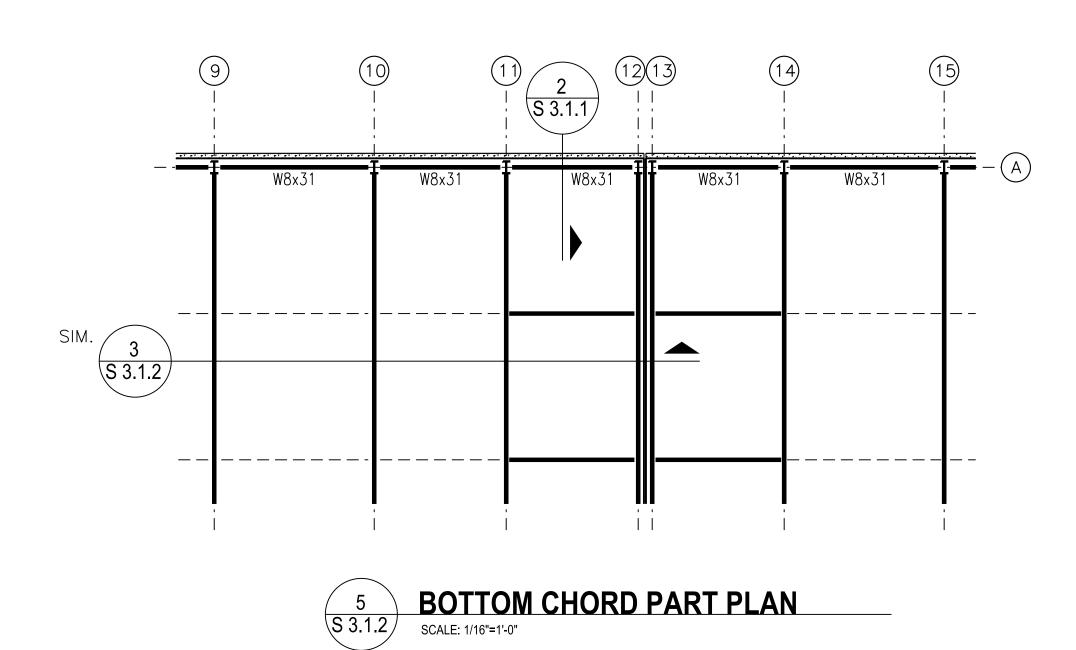


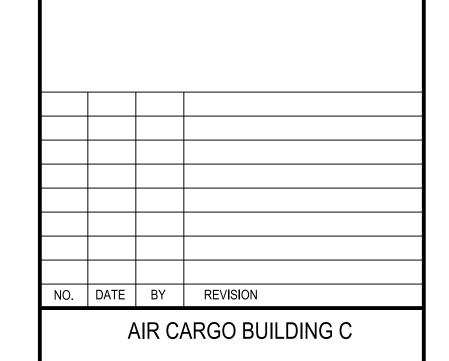












44 BROAD STREET ATLANTA, GA 30303

PHONE:404.522.3801

FAX: 404.522.3823

STEVENS & WILKINSON, INC. 100 PEACHTREE STREET NW, SUITE 2500

ATLANTA, GA 30303 PHONE: 404.522.8888 FAX: 404.521.6204

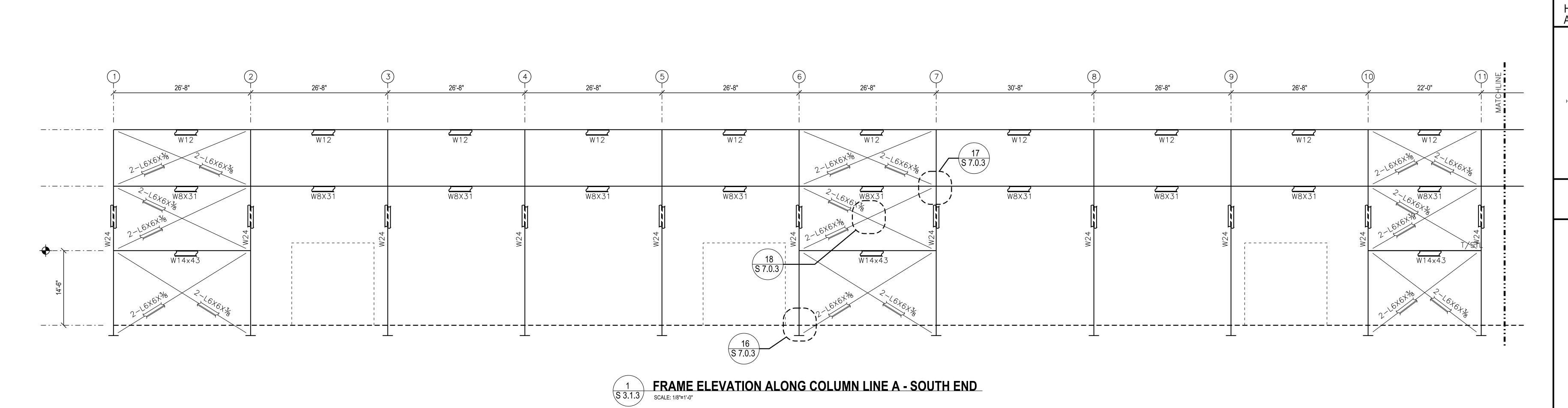
SOUTHEASTERN ENGINEERING, INC. (SEI)

2470 SANDY PLAINS ROAD MARIETTA, GA 30066

PHONE: 770.321.3936 FAX: 770.321.3935

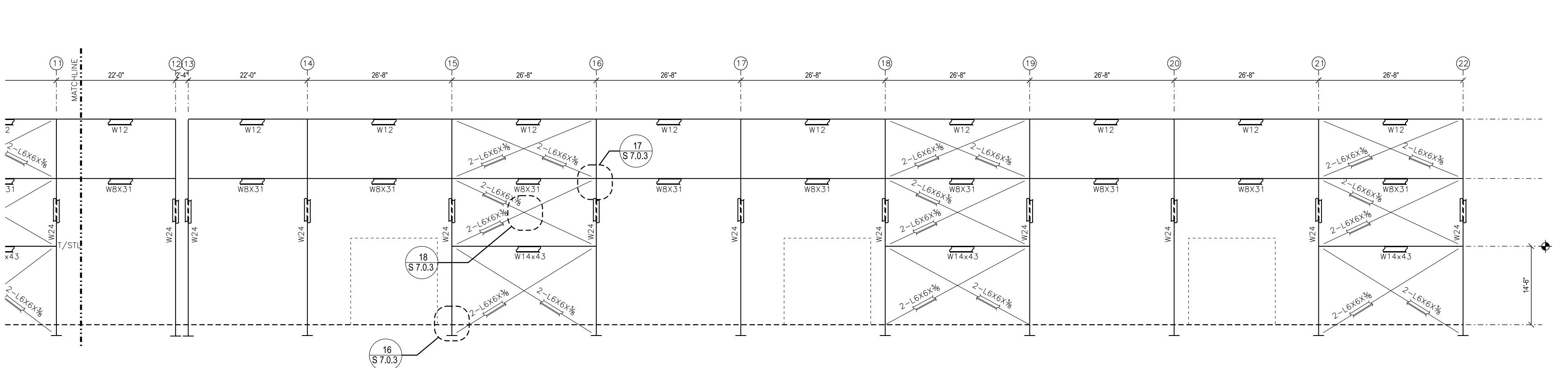
STRUCTURAL -TRUSS ELEVATION

WBS NUMBER:	DRAWN BY:	
D.07.55.009	CS	
FC NUMBER:	DESIGNED BY:	
FC-6006007929-A	MR	
A/E PROJECT NUMBER.	CHECKED BY:	
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	BP	
	DATE:	
	11/25/2014	
	SCALE:	
	AS NOTED	
	SHEET NO:	
	S 3.1.2	



1. ALL BRACING & BRACING CONNECTIONS DESIGN TO RESIST AN AXIAL FORCE OF T=C=15k (SERVICE).

2. ALL GUSSET PLATES TO BE ONE THICKNESS.
3. SPACING BETWEEN DOUBLE ANGLES TO MATCH GUSSET PLATES.
4. PROVIDE SPACERS BETWEEN ANGLES @ 1/4 POINTS.



FRAME ELEVATION ALONG COLUMN LINE A - NORTH END
SCALE: 1/8"=1'-0"

NOTE: 1. SEE 1/S 3.1.3 FOR ADDITIONAL INFORMATION.



Hartsfield-Jackson Atlanta International Airport

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PEACHTREE STREET, NE
ATLANTA, GA 30309
PHONE: 404.419.9190
FAX: 404.946.2017

STEVENS & WILKINSON, INC. 100 PEACHTREE STREET NW, SUITE 2500 ATLANTA, GA 30303 PHONE: 404.522.8888 FAX: 404.521.6204

44 BROAD STREET ATLANTA, GA 30303

PHONE: 404.522.3801

FAX: 404.522.3823

SOUTHEASTERN ENGINEERING, INC. (SEI) 2470 SANDY PLAINS ROAD MARIETTA, GA 30066 PHONE: 770.321.3936 FAX: 770.321.3935

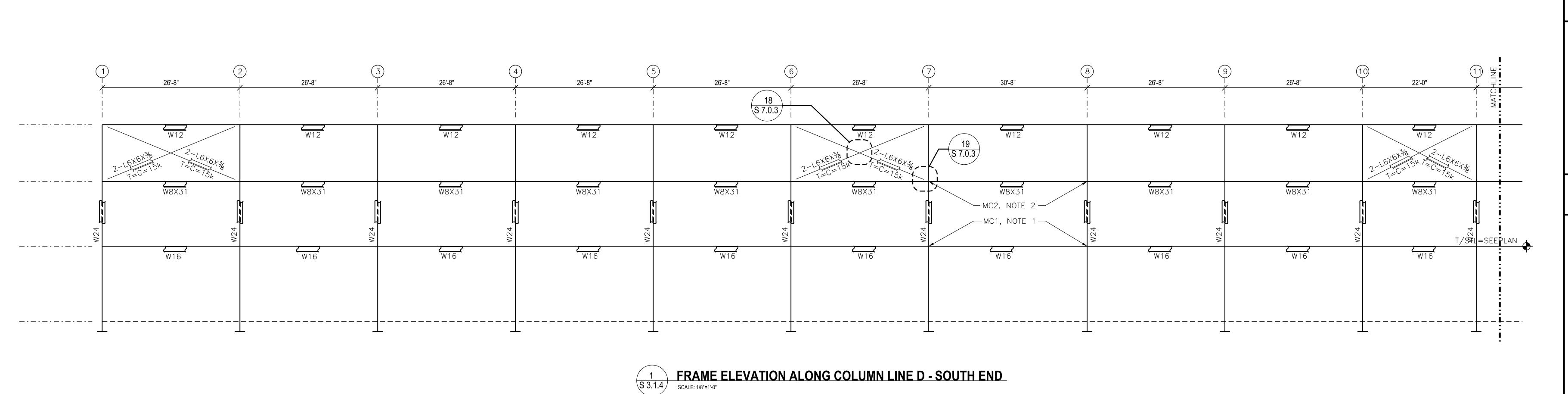
NO. DATE BY REVISION

AIR CARGO BUILDING C

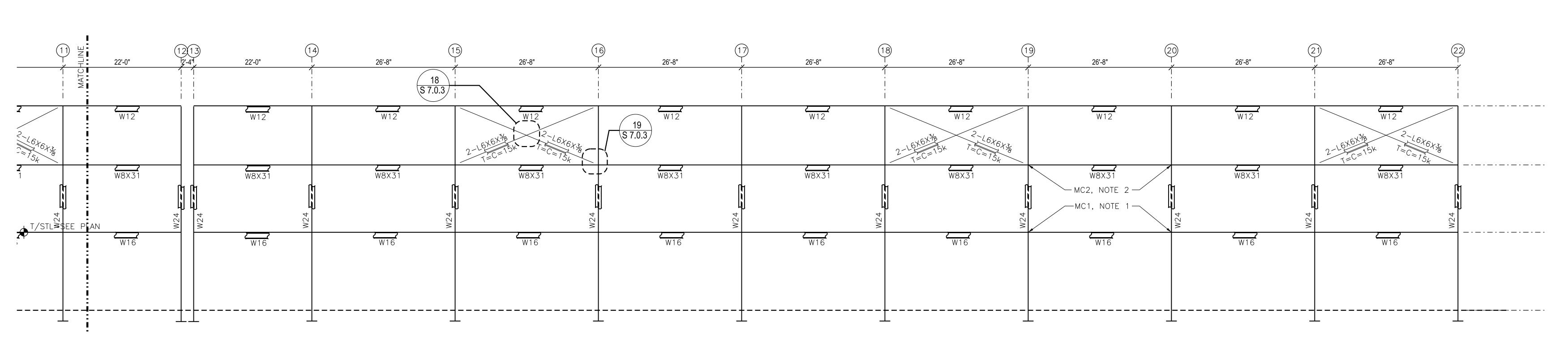
STRUCTURAL -FRAME ELEVATIONS

WBS NUMBER:	DRAWN BY:	
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FC NUMBER:	DESIGNED BY:	
FC-6006007929-A	MR	
A/E PROJECT NUMBER.	CHECKED BY:	
HII-0730621	BP	
	APPROVED BY:	
	BP	
	DATE:	
	11/25/2014	
	SCALE:	
	AS NOTED	

S 3.1.3



1. MC1 = ALL CONNECTIONS AT THIS LEVEL SHALL BE MOMENT CONNECTIONS, M=±60 FT/KIPS (SERVICE).
2. MC2 = ALL CONNECTIONS AT THIS LEVEL SHALL BE MOMENT CONNECTIONS, M=±20 FT/KIPS (SERVICE).
3. ALL GUSSET PLATES TO BE ONE THICKNESS.
4. SPACING BETWEEN DOUBLE ANGLES TO MATCH GUSSET PLATES.
5. PROVIDE SPACERS BETWEEN ANGLES @ 1/4 POINTS.





NOTE: 1. SEE 1/S 3.1.3 FOR ADDITIONAL INFORMATION.

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4.419.9190 FAX: 404.522.3823
946.2017

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NO. DATE BY REVISION

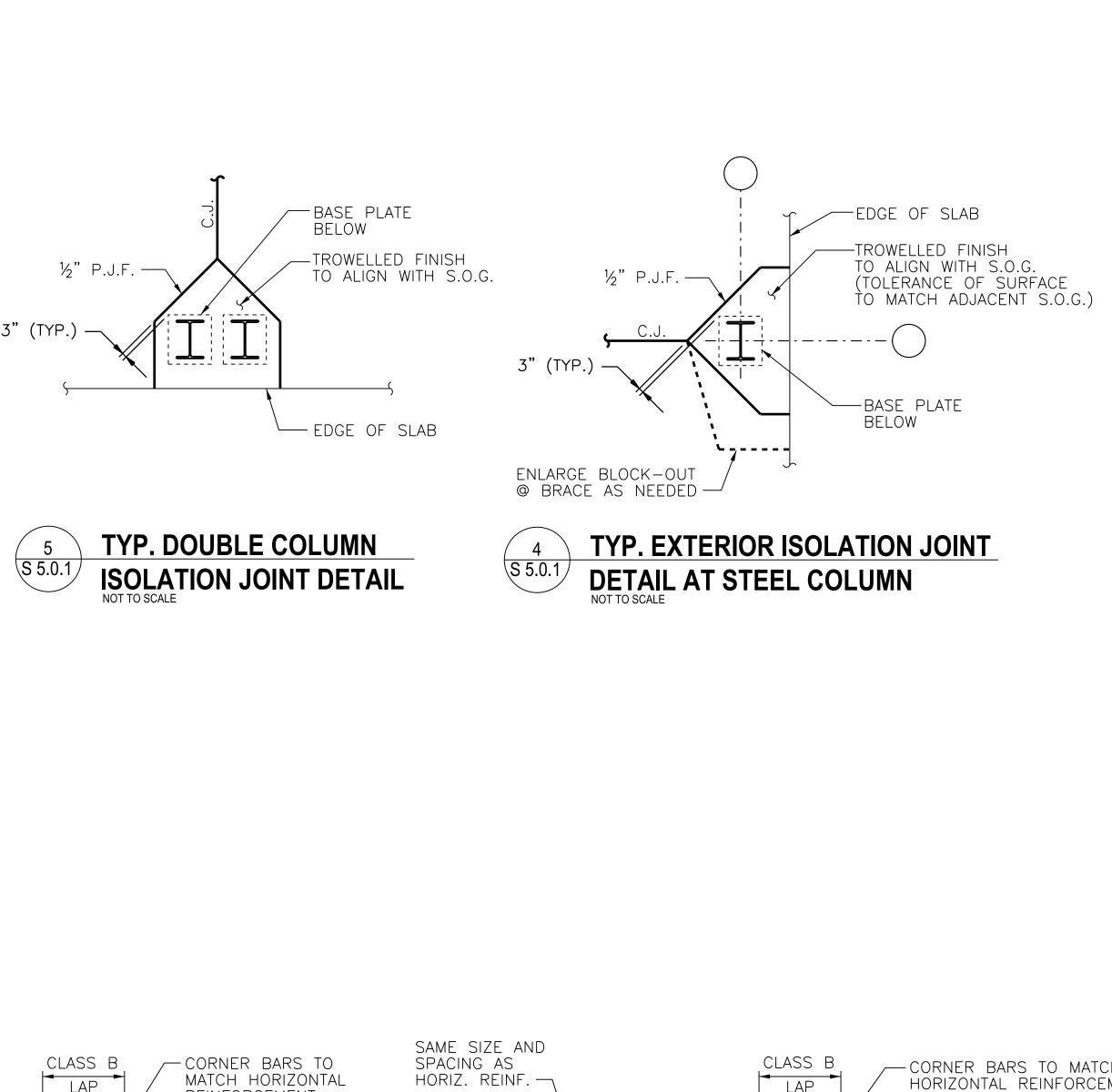
AIR CARGO BUILDING C

STRUCTURAL -FRAME ELEVATIONS

WBS NUMBER:	DRAWN BY:
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FC NUMBER:	DESIGNED BY:
FC-6006007929-A	MR
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	APPROVED BY:
	BP
	DATE:
	11/25/2014

AS NOTED

S 3.1.4



LAP

SPLICE

NOT TO SCALE

1:10 SLOPE-

**SECTION - STEPPED FOOTING** 

INDICATED ON PLAN BY THE SYMBOL 6

OTHER BARS

1'-3"

1'-7"

2'-0"

2'-5"

3'-6"

4'-0"

4'-6"

5'-1"

REINF. TO MATCH LARGER OF ADJACENT FOOTING REINF.

TYP. WALL REINFORCING (1 LAYER)

2. VERTICAL REINFORCEMENT NOT SHOWN.

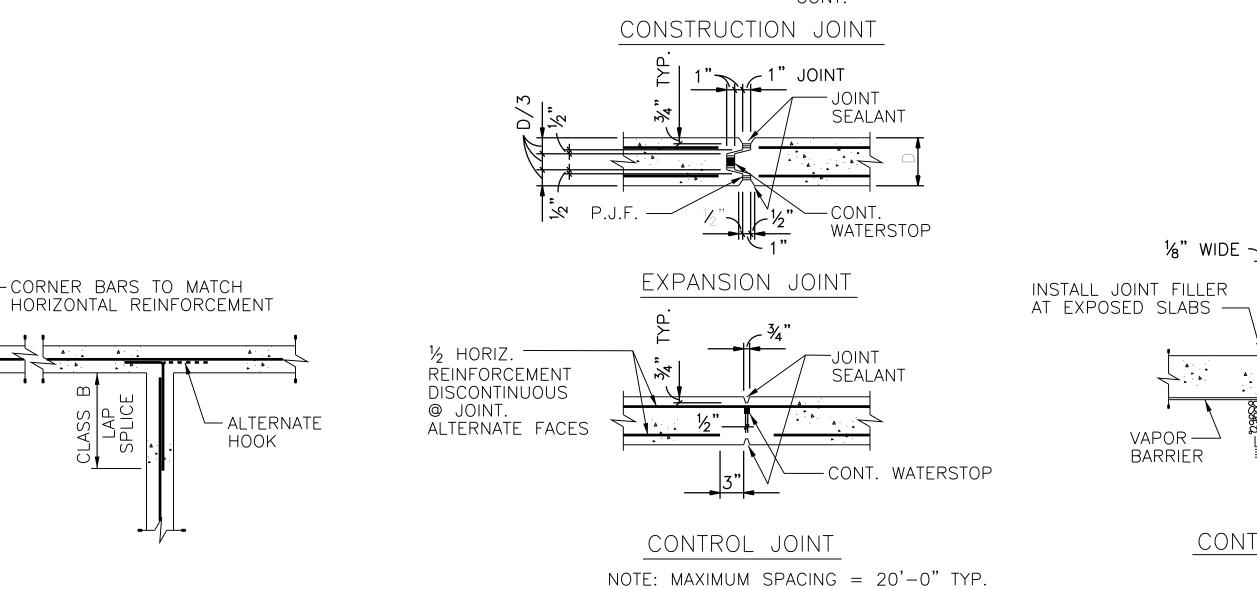
\$ SEE PLAN

4'-0" (MIN.)

1. FOR WALL REINFORCEMENT, SEE SECTIONS AND DETAILS.

1'-0" (MIN.)

SEE PLAN
FOR ELEVATION



#4@18"O.C. AT PERIMETER—

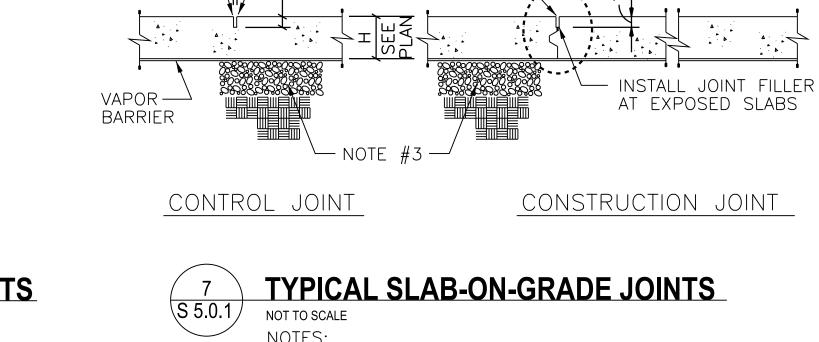
COORDINATE EXACT LOCATION AND SIZE WITH MECHANICAL/ELECTRICAL.

#4 CONT. —

√



1. ALL VERTICAL WALL REINFORCEMENT NOT SHOWN FOR CLARITY. 2. SEE SPECIFICATIONS (033000) FOR WATERSTOP INFORMATION.



SAWED JOINT

\_\_ SAWED JOINT

HEAVY HEX NUT AND PLATE WASHER

TACK

⅓" WIDE 🗽

₹ 1" U.N.O.

TYPE A

TYP. ANCHOR ROD DETAIL

1. ALL ANCHOR RODS SHALL CONFORM TO ASTM A1554, GRADE 36, U.N.O.

- HOUSEKEEPING PAD

—#3@18"O.C., E.W.

- ROUGHEN SURFACE

HOUSEKEEPING PAD ON S.O.G.

— CONC. SLAB ON GRADE, SEE PLAN



-BASE PLATE, BY PEMB SUPPLIER

-HEAVY HEX NUT

—TOP OF CONCRETE

- 2" NON-SHRINK

GROUT (MIN.)

-₽ ½"x3"x0'-3" (@ EA. ANCHOR ROD)

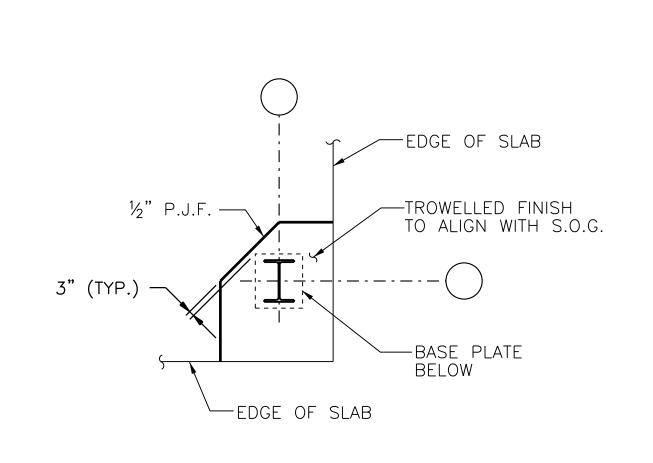
2. CONSTRUCTION JOINTS MAY BE USED IN LIEU OF CONTROL JOINTS. 3. SUB-BASE: MINIMUM OF 6" OF CLEAN (LESS THAN 6% PASSING A NO. 200 SIEVE) GRANULAR MATERIAL.

#5@12"O.C.

TYP. SLAB/ REINF. —

SEE 9/S5.0.1 FOR TYP. WALL REINF. INFO.—

SEE 17/S5.0.1 FOR WALL REINF.

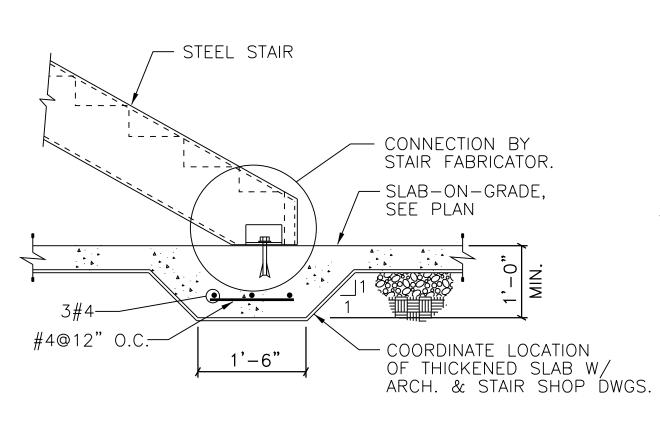


2 #4 CONT. —

**LOADING DOCK STAIR SECTION** 

COORDINATE NUMBER OF TREADS AND RISERS W/ ARCH. DRAWINGS.

TYP. CORNER ISOLATION JOINT DETAIL



2. VERTICAL REINFORCEMENT NOT SHOWN.

MATCH HORIZONTAL

**TYP. WALL REINFORCING (2 LAYERS)** 

1. FOR WALL REINFORCEMENT, SEE SECTIONS AND DETAILS.

REINFORCEMENT

SPLICE

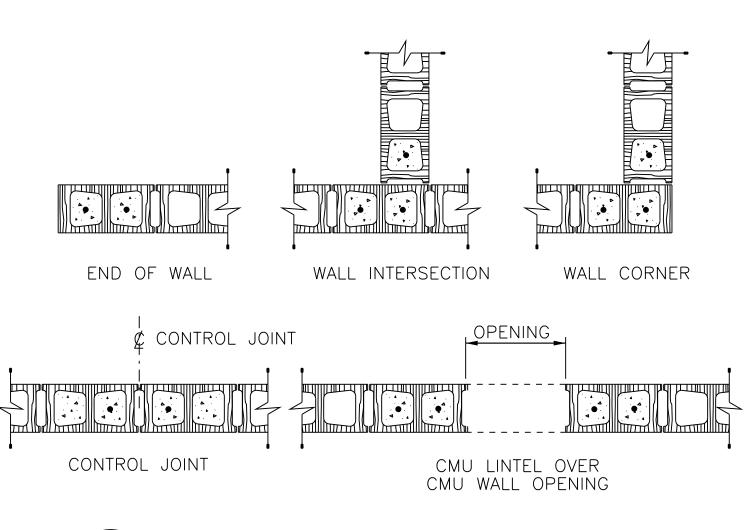
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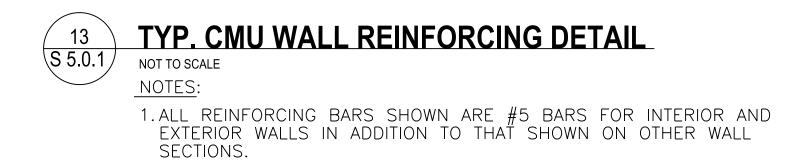


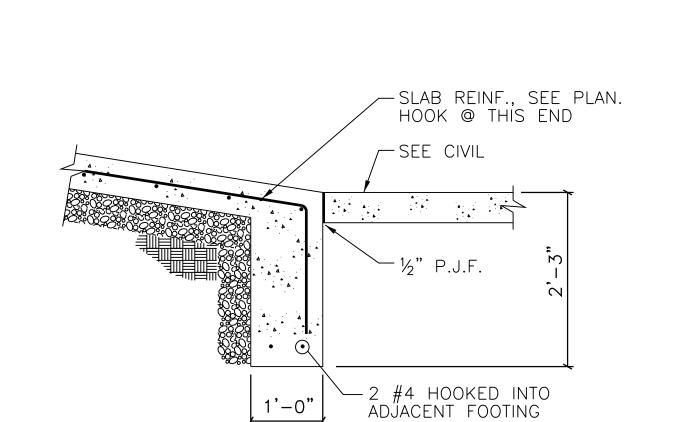
TENSION DEVE	IIMUM REINFORC ELOPMENT LENG f'c = 3000 PSI	TH SCHEDULE	TENSION DEVE	IIMUM REINFORC ELOPMENT LENG f'c = 4000 PSI	TH S
BAR SIZE	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	ОТН
#3	1'-10"	1'-5"	#3	1'-7"	
#4	2'-5"	1'-10"	#4	2'-1"	
#5	3'-0"	2'-4"	#5	2'-7"	
#6	3'-7"	2'-9"	#6	3'-1"	
#7	5'-3"	4'-0"	#7	4'-6"	
#8	6'-0"	4'-7"	#8	5'-2"	
#9	6'-9"	5'-2"	#9	5'-10"	
#10	7'-7"	5'-10"	#10	6'-7"	
#11	8'-5"	7'-6"	#11	7'-3"	

#11	8'-5"	7'-6"	#11	7'-3"	5'-7"		
LAP SPI	NIMUM REINFORC LICE LENGTH SC f'c = 3000 PSI	HEDULE	MINIMUM REINFORCING LAP SPLICE LENGTH SCHEDULE (f'c = 4000 PSI)				
BAR SIZE	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS		
#3	2'-4"	1'-10"	#3	2'-0"	1'-7"		
#4	3'-1"	2'-5"	#4	2'-8"	2'-1"		
#5	3'-11"	3'-0"	#5	3'-4"	2'-7"		
#6	4'-8"	3'-7"	#6	4'-0"	3'-1"		
#7	6'-9"	5'-3"	#7	5'-10"	4'-6"		
#8	7'-9"	6'-0"	#8	6'-8"	5'-2"		
#9	8'-9"	6'-9"	#9	7'-7"	5'-10"		
#10	9'-10"	7'-7"	#10	8'-6"	6'-7"		
#11	10'-11"	8'-5"	#11	9'-5"	7'-3"		

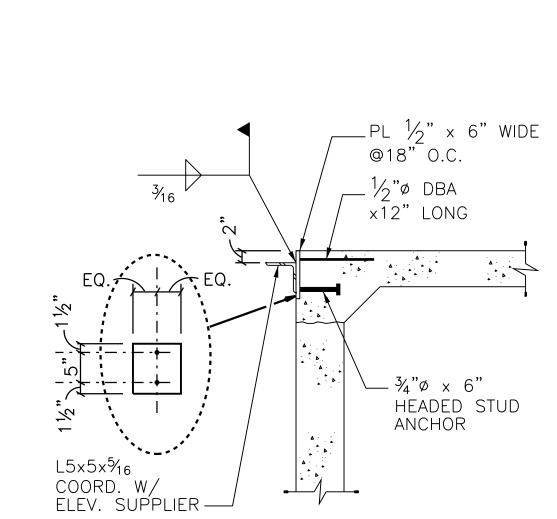
REINFORCING DEVELOPMENT AND LAP LENGTH SCHEDULES

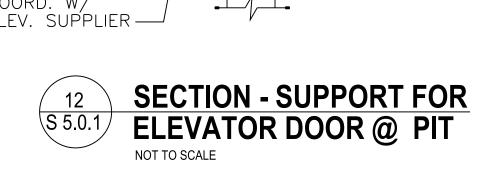


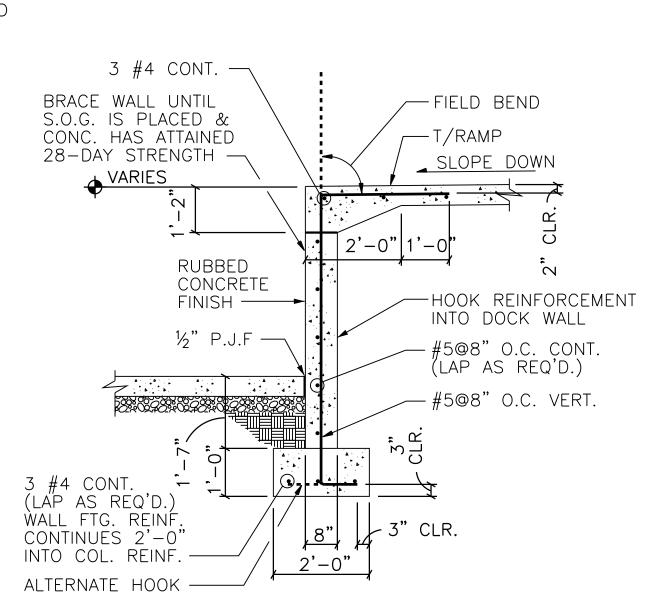




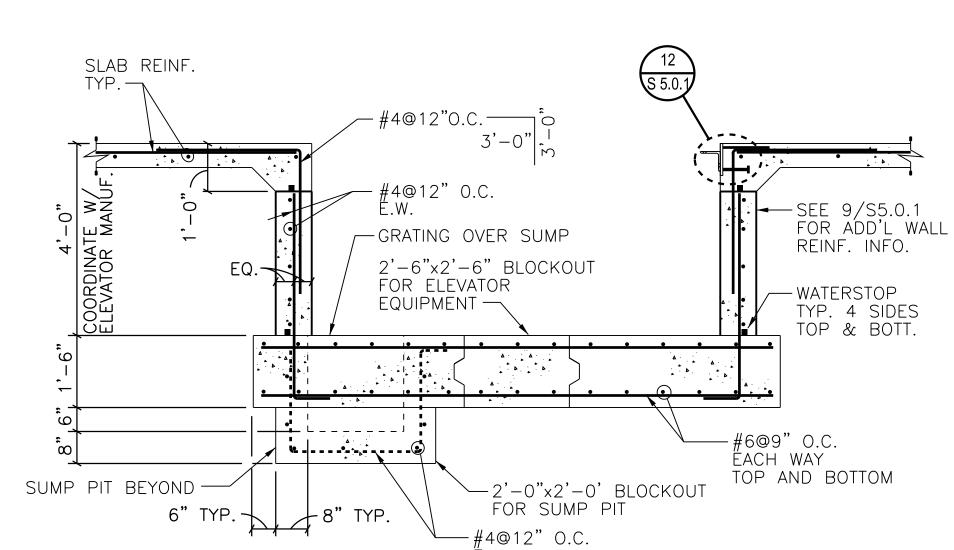




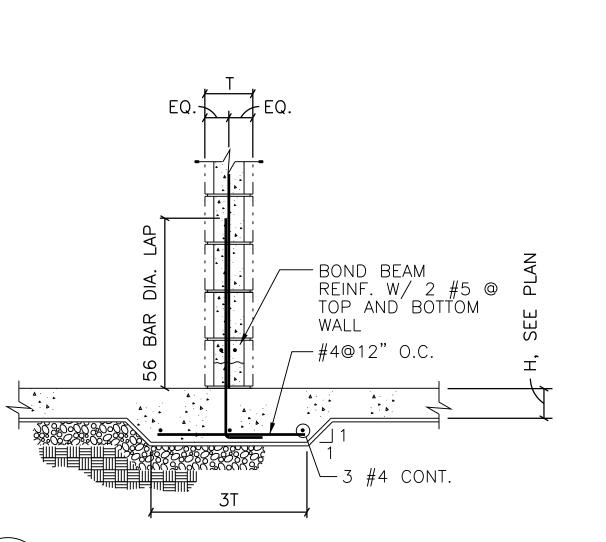




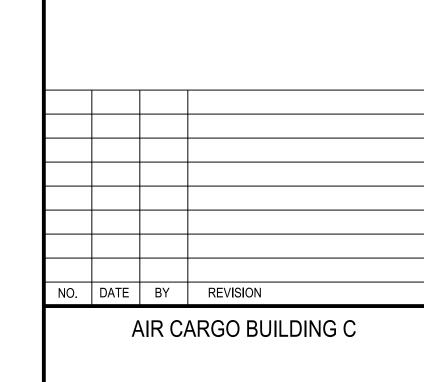








16	TYP. THICKENED SLAB @ CMU WALL
S 5.0.1	NOT TO SCALE



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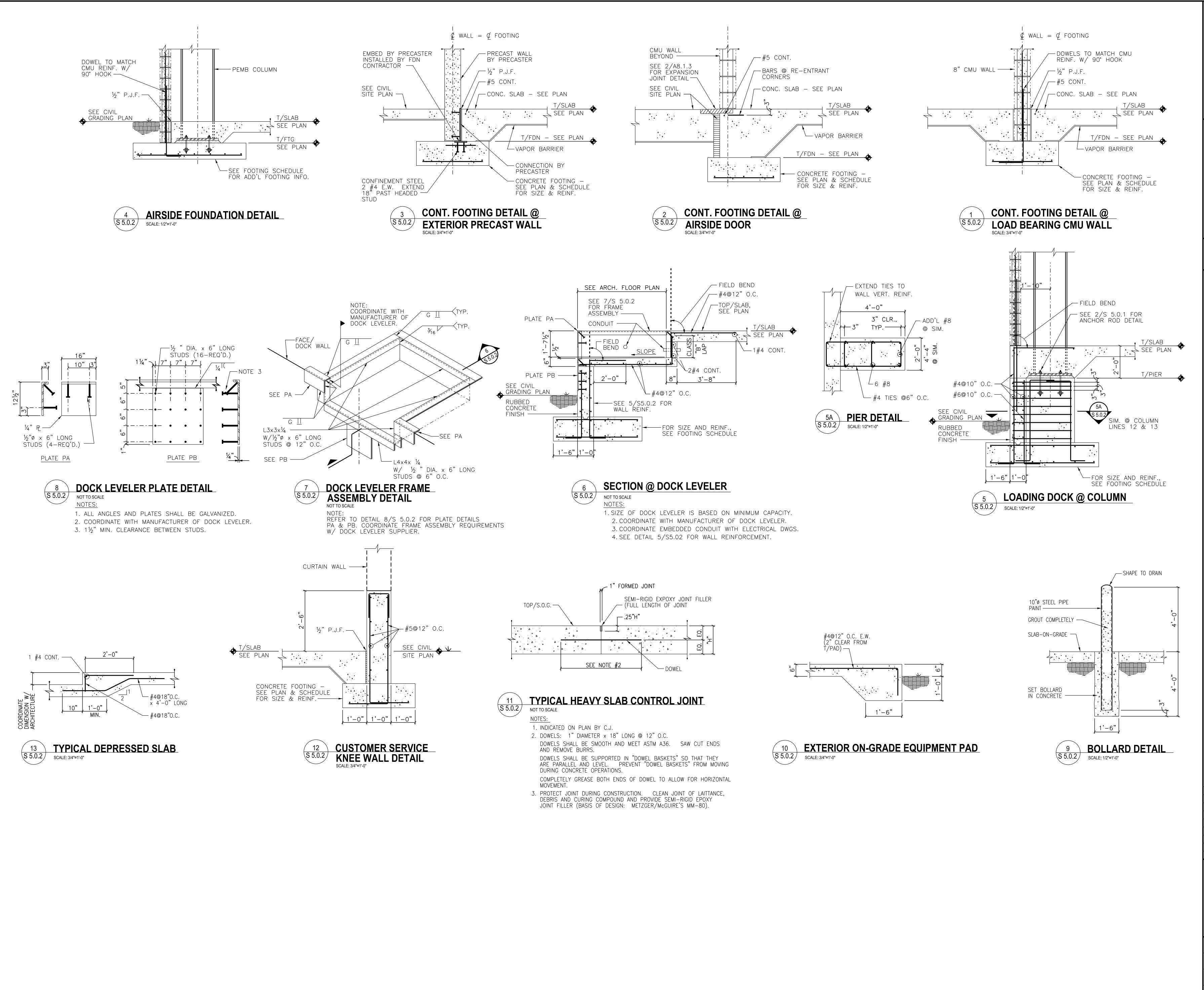
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-#4@12" O.C. (TYP.)

STRUCTURAL -	
SECTIONS AND DETAILS	

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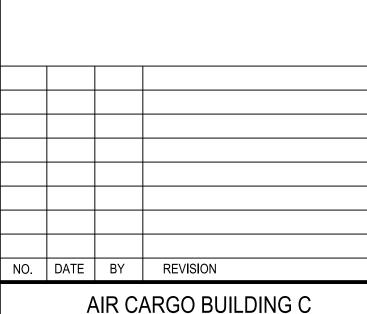
HEERY INTERNATIONAL, INC. 999 PEACHTREE STREET, NE ATLANTA, GA 30309 PHONE: 404.419.9190 FAX: 404.946.2017

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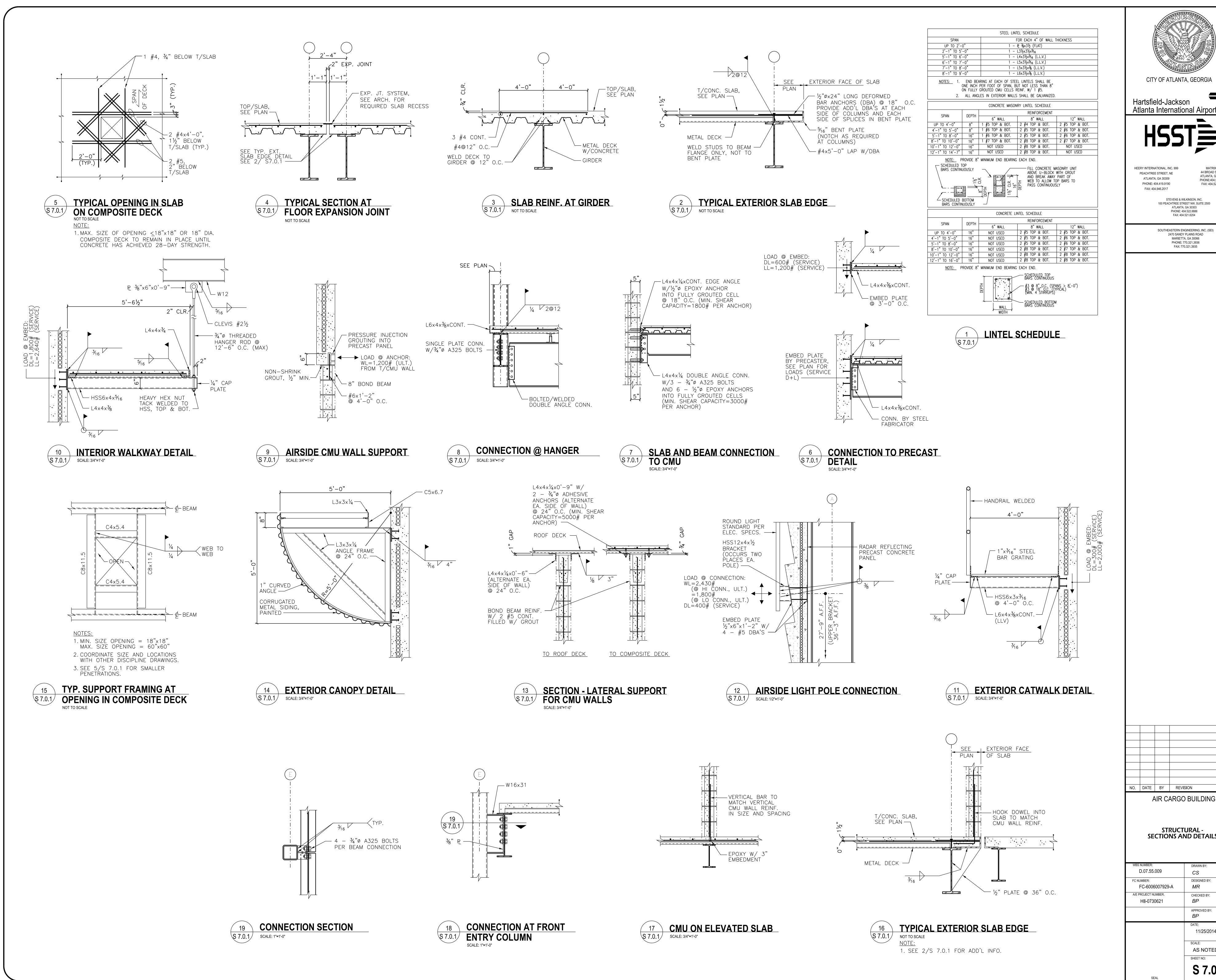
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STRUCTURAL -**SECTIONS AND DETAILS** 

D.07.55.009  FC NUMBER: FC-6006007929-A  A/E PROJECT NUMBER.	CS  DESIGNED BY:  MR  CHECKED BY:	CTION
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	AS NOTED  SHEET NO:	   RELEA
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CITY OF ATLANTA, GEORGIA

Hartsfield-Jackson Atlanta International Airport

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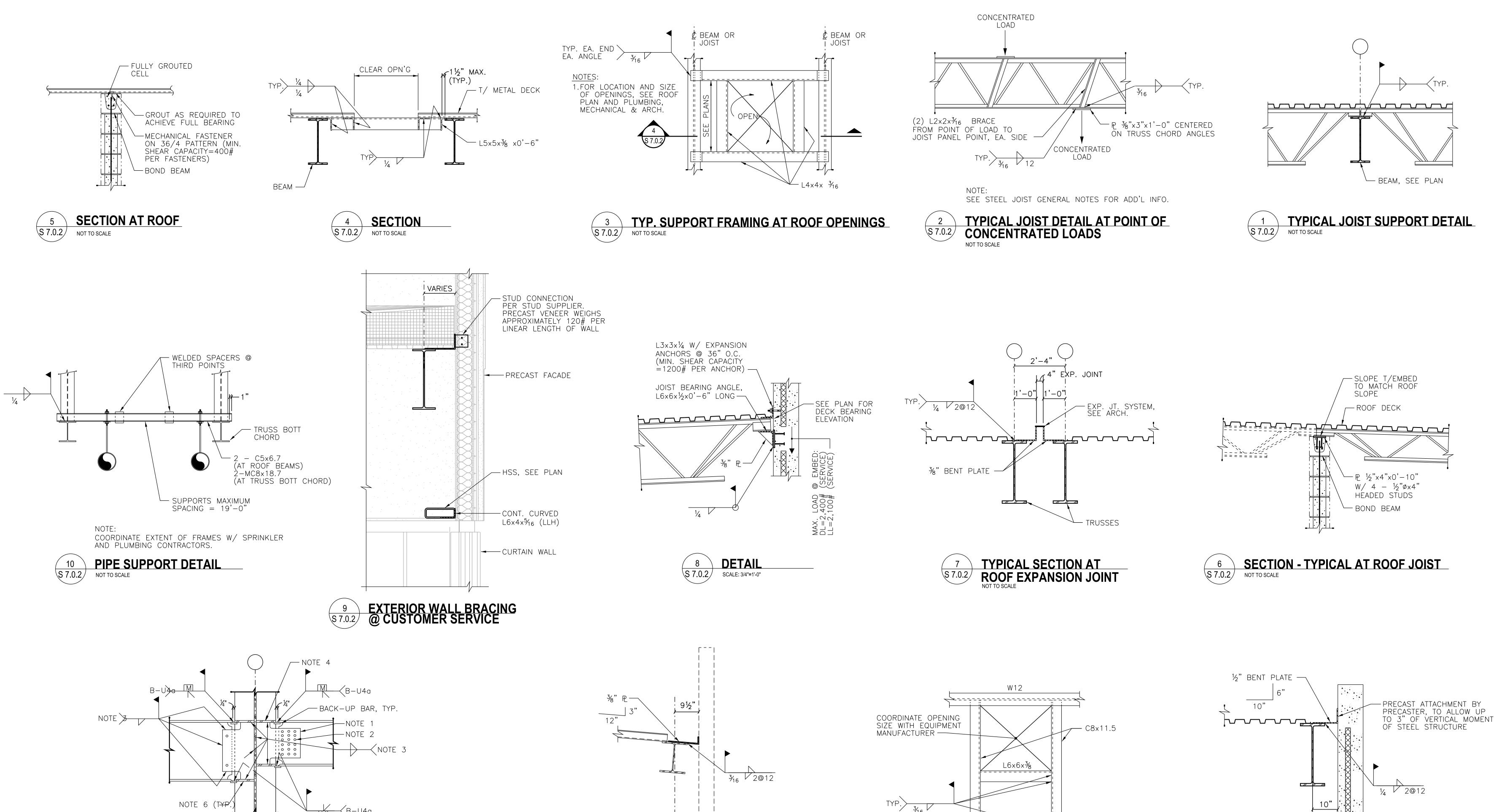
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> STRUCTURAL -**SECTIONS AND DETAILS**

CS DESIGNED BY: MR CHECKED BY: BPAPPROVED BY: BP11/25/2014 AS NOTED

SHEET NO:

S 7.0.1



LANDSIDE ROOF EDGE DETAIL

\$ 7.0.2 SCALE: 3/4"=1'-0"

WELD FOR SHEAR! BOLT FOR SHEAR

BEAM-TO-COLUMN WEB

S 7.0.2

NOT TO SCALE

BEAM FLANGES.

TYP. BEAM-TO-COLUMN MOMENT CONNECTION

1. SHEAR PLATE ONE SIDE ONLY, SIZE AS REQUIRED. OFFSET PLATE SO © OF BEAM END OF COLUMN COINCIDE, UNLESS NOTED OTHERWISE.

3. SIZE WELD TO TRANSFER SHEAR INCLUDING EFFECT OF ECCENTRICITY.

5. WHERE "A" IS 3" OR LESS, PROVIDE A SINGLE STIFFENER PLATE.

PLATE SHALL BE SAME GRADE OF STEEL AS BEAM.

4. PROVIDE STIFFENER PLATES AS REQUIRED TO FULLY DEVELOP CAPACITY OF

6. STIFFNER PLATE THICKNESS SHALL BE INCREASED 1/2" OVER MATCHING BEAM

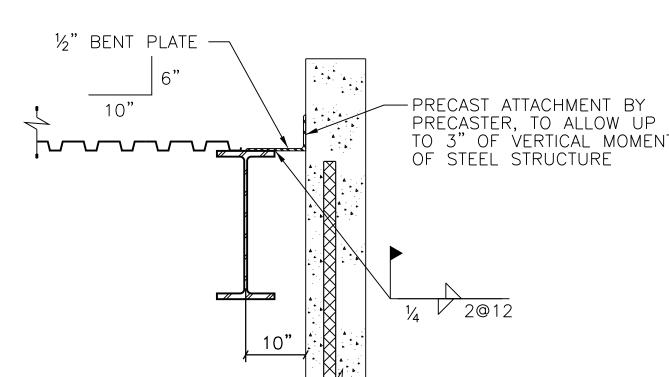
FLANGE TO ACCOMODATE MILL VARIATIONS IN BEAM DEPTH AND OUT-OF-SQUARE.

2. A325 TYPE BOLTS, SIZE AND QUANTITY AS REQUIRED TO TRANSFER SHEAR.

CONNECTION SHALL BE SLIP—CRITICAL TYPE WITH ALL BOLTS FULLY TENSIONED.

CONTACT SURFACE OF BOLTED PARTS SHALL BE CLASS A. SHORT SLOTTED

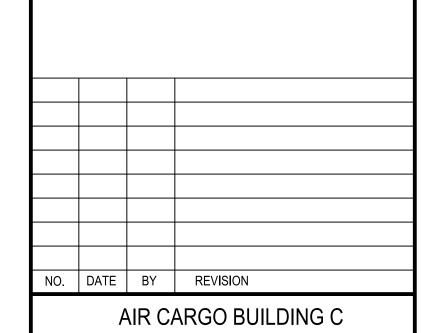
HOLES ARE PERMITTED IN SHEAR PLATE ONLY. BEAMS NOT REQUIRING ALL ROWS TO BE COMPLETELY FILLED SHALL HAVE BOLTS OMITTED AT GAGE MOST DISTANT FROM COLUMN FACE. ALL OPEN HOLES SHALL BE FILLED WITH BOLTS.



**DETAIL** S 7.0.2 SCALE: 3/4"=1'-0"



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